



Land at West Sompting,
West Sussex, BN15 0DQ

Phase 3
Drainage Report

Prepared on behalf of
Persimmon Homes
Thames Valley Limited

MILESTONE
TRANSPORT PLANNING

Land at West Sompting, West Sussex, BN15 0DQ - Phase 3 Residential Development

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1. Introduction

Background

- 1.1 Milestone Transport Planning (MTP) have been commissioned by Persimmon Homes Thames Valley Limited (the 'Client'), to prepare a Technical Note with regards to the Site situated on land to the South of Loose Lane, West Sompting to accompany a Reserved Matters Application (RMA) for Phase 3 of the development. An illustrative Site Layout Plan is contained within **Appendix A**.
- 1.2 This document should be read in conjunction with the Flood Risk and Drainage Strategy report (2017/D1358/FRA 1.4), as prepared by RGP Design Ltd, dated 06 February 2019 (contained within **Appendix C**). This Drainage Strategy has been prepared in light of the information contained within the abovementioned report, and in recognition of additional ground investigation works undertaken since the above report was produced. The additional investigation undertaken by:
- IBEX Geotechnical between October 2021 and end of January 2022.
 - Soils Ltd January 2026
- 1.3 This Technical Note aims to illustrate how the proposed RMA design accords with the Drainage Strategy and Flood Risk Assessment (FRA), prepared by RGP Design Ltd for the Outline Planning Submission, with regards to the 277 residential unit proposed for Phase 3 only.

Site Location and Description

- 1.4 The Site is located approximately 2 kilometres (km) west of Lancing, 3.6km east of Worthing, 8.6km west of Shoreham-by-Sea, and 19km west of Brighton.
- 1.5 Phase 3 of the Site is situated on land to the south of Loose Lane in West Sompting, West Sussex, and extends to approximately 12.16 hectares (ha).
- 1.6 The existing site comprises predominately of land used for agriculture. There are residential and commercial properties on the various roads adjacent to the Site.
- 1.7 As identified within the approved RGP Flood Risk Assessment, ref: 2017/D1358/FRA 1.4, the Site is classed as a 'More Vulnerable' development, with the Site being located within Flood Zone 1. The extents of flooding in relation to the Site, is illustrated on drawing "612-0510 - Flood Zone Mapping Plan contained within **Appendix G**.
- 1.8 The Site is predominately underlain by Tarrant Chalk Member, a permeable stratum accompanied by varying levels of groundwater levels.

Development Proposals

- 1.9 The Site is subject to a Hybrid planning application. (1) Outline planning permission (all matters reserved with the exception of access) for the erection of up to and including 520 dwellings, community orchard/growing space, non-vehicular links between the site and Worthing, traffic calming measures, associated landscaping, open space / recreation areas, provision of formal playing pitches, ecological enhancements including improvements and extension to Cokeham Brooks Local Wildlife Site, and all other associated development works. (2) Full planning permission for up to 278 dwellings and other associated development works (Phase 3).

- 1.10 Phase 3 of the development is located to the south of Loose Lane covering an area of circa. 12.16ha and is bounded by Phase 2 to the north, Ullswater Road to the east, the railway line to the south and the residential developments located off Dominion Way to the west.

2. Ground Conditions

- 2.1 Investigation works were undertaken for both Phases 2 and 3 of the development in October 2021 and April 2022 by IBEX Consulting Engineers. Fourteen shallow infiltration tests in accordance with BRE Digest 365 were undertaken and excavated to a maximum depth of 2.05m BGL. Within Phase 3, eight trial pits (TP07 to TP14), see Figure 2.1, were excavated to depths of 1.05m to 2.05m BGL to allow infiltration testing to be carried out. The details of this work including logs and infiltration testing are contained in the letter report dated 22/10/2021 which is included within **Appendix B** of this report.
- 2.2 The underlying geology was identified in pits TP07 to TP14 as consisting of 'Head deposits with gravelly sandy Clay' and 'Head Deposits/ Tarrant Chalk Member'.

Figure 2.1 Ibex Consulting Trial Pit Locations



Table 2.1 Ibex Consulting Infiltration rates with lowest results highlighted

Reference	Geology	Calculated Infiltration Rates (m/s)		
		1 st Cycle	2 nd Cycle	3 rd Cycle
TP07	Head Deposits	5.7x10 ⁻⁵	2.5x10 ⁻⁵	1.9x10 ⁻⁵
TP08	Head Deposits	5.8x10 ⁻⁶	1.3x10 ⁻⁵	1.4x10 ⁻⁶
TP09	Head Deposits	1.1x10 ⁻⁶	2.7x10 ⁻⁶	1.1x10 ⁻⁶
TP10	Head Deposits	1.7x10 ⁻⁶	3.0x10 ⁻⁶	*
TP11	Head Deposits	1.5x10 ⁻⁶	2.3x10 ⁻⁶	*
TP12	Head Deposits	1.0x10 ⁻⁵	2.3x10 ⁻⁵	1.9x10 ⁻⁵
TP13	Head Deposits/ Tarrant Chalk Member	8.7x10 ⁻⁴	6.5x10 ⁻⁴	1.0x10 ⁻³
TP14	Head Deposits	4.3x10 ⁻⁵	3.1x10 ⁻⁵	2.2x10 ⁻⁵

*Test collapsed following heavy rainfall.

- 2.3 In addition to the infiltration testing, IBEX also undertook winter groundwater monitoring between October 2021 and January 2022. The results of this work are contained in the Supporting Statement letter report dated April 2022, and is included as **Appendix B**.
- 2.4 The Phase 3 ground water monitoring was carried out in eight Window Sample (WS) boreholes, WS07 to WS14. Boreholes WS07, WS08 and WS10 were recorded as dry on all monitoring dates. The remaining boreholes confirmed the presence of groundwater, with levels recorded during the monitoring period.
- 2.5 **Table 2.2** below summarises the condition of the boreholes, together with the highest groundwater level recorded from the seven monitoring rounds undertaken.

Table 2.2 Ibex Consulting Winter Ground Water Monitoring Results 2022

Window Sample Location	Depth of Test Hole (m)	Peak (Highest) Ground Water Level (mBGL)	Peak (Highest) Ground Water Level (mAOD)
WS07	5.0	Dry	-
WS08	5.0	Dry	-
WS09	5.0	4.51	1.960
WS10	4.3	Dry	-
WS11	5.0	4.78	1.890
WS12	5.0	4.66	3.025
WS13	5.0	4.02	2.885
WS14	5.0	3.85	3.470

2.6 Further ground water monitoring was carried out on Site from October 2025.

2.7 **Table 2.3** below summarises the condition of the boreholes, together with the highest groundwater level recorded from the seven monitoring rounds undertaken.

Table 2.3 Ibex Consulting Winter Ground Water Monitoring Results 2025

Window Sample Location	Depth of Test Hole (m)	Peak (Highest) Ground Water Level (mBGL)	Peak (Highest) Ground Water Level (mAOD)
WS07	Borehole Destroyed		
WS08	5.01	Dry	-
WS09	5.17	4.20	2.270
WS10	Unable to Locate		
WS11	4.34	3.77	2.900
WS12	5.15	2.62	5.065
WS13	Unable to Locate Borehole		
WS14	Unable to Locate Borehole		

2.8 The results of the IBEX testing, in conjunction with the results of the latest testing, were used in the design of the on-plot infiltration systems proposed for the Phase 3 Surface Water Drainage design.

3. Drainage Strategy

Surface Water

Existing

- 3.1 Public surface water sewer records obtained from Southern Water (refer to **Appendix D**) confirm that there are no public surface water sewers located within or in the immediate vicinity of the Phase 3 development site boundary.
- 3.2 It is understood that the Site is drained partially by infiltration, whilst surface water is also discharged to the exiting watercourse(s) located to the east and south-west of Sompting South, via naturally occurring overland flow routes.

Development Proposals

- 3.3 Phase 3 of the development encompasses an area of approximately 12.16ha.
- 3.4 The proposed development retains areas of existing green space, which will be preserved as public open space through the use of pocket parks and other landscaping features. These areas have been excluded from the developable area calculations.
- 3.5 An assessment of the total impermeable area post-development has been undertaken, with the results presented on Drawing “612-0519 - Phase 3 Overall Permeability Plan”, contained within **Appendix G**. A summary of the impermeable areas is also provided below in **Table 3.1**.
- 3.6 West Sussex County Council (as Lead Local Flood Authority) has published updated guidance on ‘Policy for the Management of Surface Water’ dated 27 November 2018.
- 3.7 In accordance with the West Sussex County Council guidance, an allowance of 10% has been applied to the total impermeable area to account for urban creep. This allowance has been incorporated within each individual catchment, as detailed in the table below.

Table 3.1 Summary of Local Amenities

Catchment Type	Catchment Area	Catchment Area + 10% Urban Creep Allowance
Permeable Paving	14450	15895
Roofs	14290	15719
Highways	14970	N/A

Proposed Strategy

- 3.8 The principles established within the approved Flood Risk Assessment (FRA) prepared by RGP Design Ltd have been incorporated into the reserved matters design. Accordingly, Phase 3 of the development will be served by a gravity-fed piped drainage network. This network will incorporate features such as permeable paving and will ultimately discharge into an infiltration basin and a cellular infiltration tank.
- 3.9 The Surface Water Drainage Strategy (SWDS) has been developed in line with these principles, ensuring an integrated and sustainable approach to surface water management across Phase 3 of the site.

3.10 As a result, Phase 3 will be entirely drained using infiltration-based methods and techniques. These include:

- Private drives, private estate roads and parking - permeable paving;
- Roofs to buildings - individual or shared house soakaways/ infiltration trenches;
- Adoptable roads - piped networks outfalling to cellular soakaways and an infiltration basin.

3.11 The Surface Water Drainage modelling has been undertaken using the following criteria:

- FEH-22 Rainfall Data.
- Summer and Winter CV value of 1.
- Climate change allowance of 40% for the 1 in 10, 1 in 30 and 1 in 100-year storm events.
- Additional 10% area allowance for areas susceptible to urban creep (plot roofs and private driveways).
- No flooding for all events up to and including the 1 in 100yr+40%CC.
- No flooding for the 1 in 100yr+40%CC followed by 1 in 30yr+40%CC consecutive storms for the Infiltration Basin.

3.12 The results of this modelling are provided within **Appendix E** of this report.

Permeable Paving

3.13 Within Phase 3 of the development, it is proposed that permeable paving will be introduced for the non-highway adopted paved areas, comprising of private estate roads, shared parking areas and private driveways.

3.14 The total plan area for the extents which are proposed to be constructed using permeable features is 15,895m², inclusive of a 10% allowance for urban creep. Calculations contained within **Appendix E** of this Report confirm that a varying depth sub-base with a void ratio of 30% will provide sufficient storage and allow adequate infiltration to drain the impermeable areas.

3.15 In view of the additional infiltration testing undertaken by IBEX Consulting at shallow depths as associated with traditional permeable paving construction, consideration will be afforded to the varying rate(s) of infiltration established through the additional testing undertaken. The 'catchments' associated with each infiltration rate established by IBEX Consulting will be related to the proposed site layout to ensure that assessment and calculations are accurate and are a fair reflection of the associated soils' abilities where infiltration is concerned.

House Soakaways

3.16 The total roof area associated with the proposed development is 15,719 m², inclusive of a 10% allowance for urban creep. An assessment has been undertaken, taking into account the variable infiltration rates identified at shallow depths across the site, to evaluate the suitability of soakaways for draining runoff from these roof areas.

3.17 For this assessment, the Site has been separated into five indicative infiltration areas. A conservative approach has been adopted within each area by identifying the residential plots with the greatest proposed building footprints and using these as the basis for the assessment.

3.18 The hydraulic calculations for each catchment have therefore been calculated using the largest anticipated plot coverage, together with the lowest recorded infiltration rate for that area. This is in order to reflect the most conservative design scenario.

- 3.19 The calculations presented in **Appendix E** provide an indication of the storage requirements likely to be needed for each plot. The final drainage provision will be established during the detailed design stage once the appropriate infiltration characteristics for each plot have been confirmed. A summary of the indicative sizes of the house crate soakaways, is also provided below in **Table 3.2**.

Table 3.2 Indicative House Crate Soakaway Details

House Soakaway ID	SA1/1	SA2/1	SA3/1	SA4/1
Drained Area (m ²)	81	92	82	80
Urban Creep Allowance (+10%) (m ²)	90	102	91	88
Depth of Cover (m)	0.8	0.8	0.8	0.8
Infiltration Rate (m/hr)	0.0792	2.34	0.0396	0.00504
Indicative Soakaway Size (m ² x D)	2.0m x 2.0m x 2.0m	1.5m x 1.5m x 1.5m	2.0m x 3.0m x 2.0m	2.0m x 5.0m x 2.0m
Volume (m ³)	7.60	3.206	11.40	19.00

- 3.20 It should be noted that further infiltration testing, in accordance with BRE365, will be undertaken during detailed design. This will then be used to inform individual house soakaway designs for each development plot.

Adoptable Roads

- 3.21 It is understood that both the main access and spine road, present throughout the phase 3 development, will be offered for adoption to the local Highways. The access roads and public footways will be drained via a positive, piped drainage system, which will outfall into an infiltration basin and a cellular attenuation tank.
- 3.22 The Surface Water Drainage Strategy (SWDS) has been divided into two discrete drainage networks to effectively manage surface water runoff across the Site closer to the source.
- 3.23 Network 1 comprises a conventional piped drainage system that collects runoff from the contributing catchment and conveys it to an underground cellular attenuation tank. The cellular tank is designed to provide temporary storage of stormwater during rainfall events and subsequently discharge flows via infiltration into the underlying ground through a soakaway arrangement.
- 3.24 Network 2 consists of a separate piped drainage network that conveys runoff directly to an infiltration basin. The basin provides both attenuation and infiltration functions, allowing surface water to be stored temporarily before infiltrating into the ground at a controlled rate.
- 3.25 The two-network arrangement has been developed in response to site constraints, topography, catchment distribution and infiltration opportunities, ensuring that surface water runoff is managed efficiently and in accordance with the relevant drainage standards and SuDS best practice guidance.
- 3.26 The drainage strategy has been designed to accommodate a 1 in 100-year storm event, with the inclusion of an allowance of 40% for climate change.
- 3.27 The Surface Water Drainage proposals are shown on drawings “612-0500 to 09 inclusive and are contained within **Appendix G** of this report.

3.28 Supporting calculations are also contained within **Appendix E**.

- **Infiltration Basin (Network 2)**

3.29 The infiltration basin has been sized to accommodate the above criteria and rainfall events, and the design properties are summarised in **Table 3.3** below.

3.30 The infiltration basin has been designed with a maximum side slope of 1:3 and have a minimum freeboard of 0.3m as recommended within the CIRIA SuDS Manual.

3.31 Infodrainage calculations/ results are provided within **Appendix E** of this report.

Table 3.3 **Infiltration Basin Details**

Infiltration Basin ID	ATTN BASIN (pond)
Cover Level	6.530mAOD (min.)
Basin Bed Level	3.800mAOD
Freeboard	300mm
Side Slopes	1:3 maximum
Top Area	4746.50m ²
Base Area	2715m ²
Maximum Depth	3.159m
Total Volume	8367.832m ³
Infiltration Rate (m/hr)	0.00396

- **Geocellular Tank Soakaway (Network 1)**

3.32 The proposed geocellular tank soakaway has been sized to accommodate the required design criteria and rainfall events. The key design parameters are summarised in **Table 3.4** below.

3.33 Hydraulic modelling confirms that the soakaway provides sufficient storage capacity and performs satisfactorily for the assessed storm events.

3.34 Causeway Flow calculations/ results are provided within **Appendix E** of this report.

Table 3.4 Geocellular Tank Soakaway Details

Soakaway ID	ATTN TANK 7
Cover Level (min.)	6.897mAOD
Depth of Cover	1.074mAOD
Top of Tank	5.823mAOD
Tank Invert	4.323mAOD
Soakaway Size (m ² x D)	153m ² x 1.5m
Volume (m ³)	229.5
Infiltration Rate (m/hr)	2.34

- 3.35 Freeboard has been provided in view of the 'peak' (highest) groundwater levels established within the vicinity of the proposed soakaway locations. The locations for monitoring were selected in view of the anticipated location(s) of the soakaway features, albeit in relation to an earlier revision of the current site arrangement, which has since developed to the revision that is contained within this report.

CIRIA Pollution Mitigation Indices

- 3.36 The Site is categorised as a 'Low' risk development and it is therefore proposed that the required pollution mitigation will be provided through the use of both permeable paving and an infiltration Basin. **Table 3.5** and **Table 3.6** set out the pollution hazard and mitigation performances of each component in relation to the required indices for the development.

Table 3.5 Pollution Hazard Indices, Extracted from Table 26.2 CIRIA C753

Pollution Hazard Level	Total Suspended Solids (TSS)	Metals	Hydrocarbons
Low (Residential parking and general access roads)	0.5	0.4	0.4

Table 3.6 Pollution Mitigation Indices, Extracted from Table 26.3 and 26.4 CIRIA C753

Pollution Mitigation Measure	Total Suspended Solids (TSS)	Metals	Hydrocarbons
Permeable Pavement	0.6	0.7	0.6
Pond	0.7	0.7	0.5

Overland Flow and Exceedance Routes

- 3.37 As illustrated within the Overland Flow Plan, contained within **Appendix G**,
- 3.38 Levels have been designed such that properties would sit above the proposed highway wherever possible to minimise the risk of flooding.

New National Standards for SuDS

- 3.39 The following Section provides a summary of how the proposed Surface Water Drainage Strategy (SWDS) and associated Sustainable Drainage Systems (SuDS) measures, satisfy the requirements set out within the new National SuDS Standards (NSS).

Standard 1 - Runoff Destinations

- 3.40 The SWDS has been designed to discharge surface water entirely through infiltration techniques with no connections to surface water sewers or watercourses which meets priority 2 of Standard 1.

Standard 2 - Management of Everyday Rainfall (Interception)

- 3.41 The proposed SWDS will provide interception by utilising the following SuDS features:
- Infiltration Basins / Crates;
 - Soakaways; and
 - Permeable Paving.
- 3.42 The proposed drainage system will infiltrate all storms up to 1 in 100-year + 45% CC therefore, the SWDS meets the requirements of Standard 2.

Standard 3 - Management of Extreme Rainfall and Flooding

- 3.43 The proposed SuDS has been designed to manage runoff from the 1 in 100-year rainfall event, including a 40% allowance for climate change.
- 3.44 The infiltration basin has been sized accordingly and provides sufficient storage to manage runoff from all design events up to and including the 1 in 100-year plus 40% climate change event, with no flooding predicted on Site during these events. Therefore, the SWDS is compliant with Standard 3.

Standard 4 - Water Quality

- 3.45 The proposed SWDS has incorporated a SuDS management train throughout the development to provide appropriate treatment of runoff before infiltration. Surface water runoff from parking areas and private drives is directed through permeable paving systems, whilst runoff from adoptable highways is conveyed to an infiltration basin and geocellular soakaway system.
- 3.46 In line with CIRIA C753 guidance, the development is categorised as a 'Low' risk development (refer to **Section 3.63**). A pollution hazard assessment has been undertaken in line with CIRIA guidance and is presented in **Table 3.6**.
- 3.47 The proposed SuDS components incorporated within the SWDS provide pollution mitigation indices that exceed the required pollution hazard indices for total suspended solids (TSS), metals, and hydrocarbons.
- 3.48 Therefore, the SWDS provides appropriate treatment of runoff and conforms with Standard 4.

Standard 5 - Amenity

- 3.49 The Surface Water Drainage System works with and complements the wider landscape proposals for the Site to provide opportunities for amenity through multi-functional places. Therefore, the proposals meet Standard 5.

Standard 6 - Biodiversity

- 3.50 The infiltration basin and surrounding landscaped areas will help create new habitats and improve links between existing wildlife areas across the Site. By using natural drainage methods and incorporating SuDS features into the landscape, the drainage design contributes to the development's ecological goals and supports the wider ecological improvements proposed as part of the masterplan. Details of planting and other biodiversity measures will be developed during the detailed design stage. Therefore, the SWDS is compliant with Standard 6.

Standard 7 - Design of Drainage for Construction, Operation, Maintenance, Decommissioning and Structural Integrity

- 3.51 The Surface Water Drainage Strategy has been developed to ensure that the proposed drainage system can be constructed, operated, and maintained safely and efficiently throughout its design life.
- 3.52 A preliminary SuDS Management and Maintenance Plan has been prepared as part of this assessment to demonstrate the long-term viability of the proposed drainage measures.
- 3.53 As the design progresses, the Management and Maintenance Plan will be reviewed and finalised during the detailed design stage to reflect the final drainage layout and operational requirements.

Foul Water

Background Information

- 3.54 Southern Water (SW) is the Sewerage Undertaker for the area.
- 3.55 SW record plans obtained in 2021 (refer to **Appendix D**), confirmed the presence of existing public sewers within the Site Boundary and its immediate surroundings.
- 3.56 A 225mm diameter vitrified clay (VC) foul sewer extends from Sylvan Road in a south-easterly direction, crossing the Site at its north-eastern boundary. Beyond this point, the sewer connects into a 450mm diameter VC foul sewer that runs parallel to the watercourse in the east before crossing the railway line to the south.
- 3.57 Additionally, further sewerage assets are located within the neighbouring residential developments of Ullswater Road, Dominion Way and Florlandia Close.

Anticipated Foul Flows

- 3.58 As part of Pre-Development Enquiries to establish both network capacity and a viable point of connection, an outline foul water (FW) drainage strategy and supporting foul flow calculations were submitted to Southern Water in June 2026. The proposed strategy is based on a gravity connection to the existing foul sewer network located near the south-eastern boundary of the Site.
- 3.59 Peak foul flows have been calculated in accordance with Southern Water Guidance (DS-Foul Sewerage Modelling Criteria), which specifies the following residential foul flow rates:
- Dry Weather Flow - 125l/dwelling/day
 - Peaking Factor - 2.5 +0.1
 - Occupancy - 2.4 persons/dwelling
- 3.60 A copy of the submitted foul flow calculations is included in **Appendix F**.
- 3.61 A formal Pre-Development Enquiry was submitted to Southern Water in June 2026 to confirm the available network capacity and the proposed point of connection. A response is currently awaited.

Foul Drainage Strategy

- 3.62 The proposed foul drainage strategy is based on a gravity connection to the existing foul sewer network, subject to confirmation by Southern Water. The final point of connection will be agreed/ confirmed following receipt of the Pre-Development Enquiry response.
- 3.63 The Outline Foul Drainage Strategy drawings are contained within **Appendix G**.
- 3.64 A Section 106 application would be prepared and submitted to Southern Water during the detailed design stage.
- 3.65 It should be noted that, if a third-party supplier were to serve the development, an agreement would be required to transfer foul water service areas through a New Appointment and Variation (NAV). This would be considered and, if necessary, also progressed during the detailed design stage.

4. Conclusion

- 4.1 This Technical Note was commissioned by Persimmon Homes Thames Valley Limited (the 'Client'), to accompany a Reserved Matters Application (RMA) for Phase 3 of a proposed development situated on land to the South of Loose Lane, West Sompting.
- 4.2 This Technical Note has illustrated how the proposed RMA drainage design accords with the principles set out within the previously approved FRA produced by RGP Design Ltd.
- 4.3 Surface water drainage proposals have been developed in accordance with current national and local policy and guidance, incorporating sustainable drainage principles where appropriate.
- 4.4 Additional allowances for both climate change and urban creep have been incorporated into the design, in line with relevant national and local guidance.
- 4.5 Appropriate pollution control measures will be incorporated into the drainage design to ensure that surface water runoff does not adversely impact receiving water environments.
- 4.6 The foul drainage strategy is based on a gravity connection to the existing foul sewer network, subject to confirmation from Southern Water. The final point of connection will be agreed following receipt of the Pre-Development Enquiry response.
- 4.7 The relevant discharge approvals would be sought from the approving bodies during the detailed design stage.
- 4.8 Peak foul flows have been calculated in accordance with Southern Water Guidance (DS-Foul Sewerage Modelling Criteria), which specifies the following residential foul flow rates:
- Dry Weather Flow - 125l/dwelling/day
 - Peaking Factor - 2.5 +0.1
 - Occupancy - 2.4 persons/dwelling
- 4.9 A formal Pre-Development Enquiry was submitted to Southern Water in June 2026 to confirm the available network capacity and the proposed point of connection. A response is currently awaited

Drawings:

612-0500	Phase 3 Drainage Overall Plan
612-0501	Phase 3 Drainage Plan Sheet 1 of 9
612-0502	Phase 3 Drainage Plan Sheet 2 of 9
612-0503	Phase 3 Drainage Plan Sheet 3 of 9
612-0504	Phase 3 Drainage Plan Sheet 4 of 9
612-0505	Phase 3 Drainage Plan Sheet 5 of 9
612-0506	Phase 3 Drainage Plan Sheet 6 of 9
612-0507	Phase 3 Drainage Plan Sheet 7 of 9
612-0508	Phase 3 Drainage Plan Sheet 8 of 9
612-0509	Phase 3 Drainage Plan Sheet 9 of 9
612-0510	Phase 3 Flood Zone Mapping Plan
612-0519	Phase 3 Preliminary Impermeable/ Permeable Areas
612-0521	Phase 3 Infiltration Area Plan