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Infiltration Report

at
South Pavilion, Buckingham Park, Upper Shoreham Road, Shoreham by Sea, West Sussex BN43 6BA

for
Price & Myers LLP

Reference: 21463/IR Rev1.0
August 2024

Control Document

Project
South Pavilion, Buckingham Park, Upper Shoreham Road, Shoreham by Sea, West Sussex BN43 6BA

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This is not a valid document for use in the design of the project unless it is titled Final in the document status box.

Current regulations and good practice were used in the preparation of this report. The recommendations given in this report must be reviewed by an appropriately qualified person at the time of preparation of the scheme design to ensure that any recommendations given remain valid in light of changes in regulation and practice, or additional information obtained regarding the site.

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Commission

This document comprises the Infiltration Report (IR) and incorporates the results, discussion, and conclusions to this intrusive works. General site data is recorded below:

Commission Record	
Client	Price & Myers LLP
Site Name	South Pavilion, Buckingham Park, Upper Shoreham Road, Shoreham by Sea, West Sussex BN43 6BA
Grid Reference	TQ 220 059
Soils Limited Quotation Ref	Q28823, dated 25th April 2024 and email correspondence on 16th May 2024
Clients Purchase Order	PO1012705 dated 9th May 2024 and PO1012756 dated 19th May 2024

Limitations and Disclaimers

The report was prepared solely for the brief described in Section 1.1 of this report.

The contents, recommendations and advice given in the report are subject to the Terms and Conditions given in Soils Limited's Quotation.

Soils Limited disclaims any responsibility to the Client and others in respect of any matters outside the scope of the above.

This report has been prepared by Soils Limited, with all reasonable skill, care and diligence within the terms of the Contract with the Client, incorporation of our General Conditions of Contract of Business and taking into account the resources devoted to us by agreement with the Client.

The report is personal and confidential to the Client and Soils Limited accept no responsibility of whatever nature to third parties to whom this report, or any part thereof, is made known. Any such party relies on the report wholly at its own risk.

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The ground is a product of continuing natural and artificial processes. As a result, the ground will exhibit a variety of characteristics that vary from place to place across a site, and also with time. Whilst a ground investigation will mitigate to a greater or lesser degree against the resulting risk from variation, the risks cannot be eliminated.

The investigation was prepared for the sole benefit of the Client in accordance with their brief. As such these do not necessarily address all aspects of ground behaviour at the site.

Current regulations and good practice were used in the preparation of this report.

If the term "competent person" is used in this report or any Soils Limited document, it means an engineering geologist or civil engineer with a minimum of three years post graduate experience in the understanding and application of the appropriate codes of practice.

Within the report reference to ground level relates to the site level at the time of the investigation, unless otherwise stated.

Exploratory hole is a generic term used to describe a method of direct investigation. The term trial pit, borehole or window sample borehole implies the specific technique used to produce an exploratory hole.

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Section 1 Introduction

1.1 Objective of Investigation

The Client commissioned Soils Limited to undertake an infiltration investigation and to prepare an Infiltration Report to supply the Client and their designers with information regarding ground conditions and infiltration test results, to assist in the drainage design for the development that was appropriate to the settings present on the site.

No preliminary investigation, environmental/geotechnical testing or interpretive reporting was commissioned as part of this investigation.

1.2 Site Description

At the time of the works in May 2024, the area of the investigation was located at the south-western portion of the grounds of Buckingham Park at Shoreham by Sea, it was covered in grass and was gently sloping to the south in alignment with the wider topography that was noted to have a fall of circa 3 degrees (source: OS Maps) to the south towards the River Adur and the coast located 900m and 1500m to the south respectively.

The area of the investigation was surrounded by grass in its immediate vicinity with sports fields to the north and north-east and a coffee shop further to the east, with a MUGA playground to the west and Young Sussex Nursery to the south. No trees were noted within or in proximity (circa 30m) to the area of the investigation.

The site location plan is given in Figure 1. An aerial photograph of the site and its close environs has been included in Figure 2.

1.3 Proposed Development

The proposal comprised the construction of a single storey pavilion that is understood to be used as a coffee shop and changing rooms.

Development plans provided by the Client (ref: SK 6000/31383, dated 7/3/24 by Price & Myers) are presented in Appendix D.

1.4 Anticipated Geology

The 1:50,000 BGS map showed the site to be located upon the bedrock Tarrant Chalk Member with overlying superficial deposits of Head.

1.4.1 Head

Head deposits are drifts produced by solifluxion, the downslope movement of debris outwash during the periglacial period, and characteristically comprise poorly sorted sands gravels and chalk of local derivation.

1.4.2 Tarrant Chalk Member

The Tarrant Chalk Member typically comprised soft white chalk with relatively widely spaced but large flint seams. The lower boundary is conformable. Well-developed marls commonly occur for several metres above the Castle Hill Marls, up to and including the Pepper Box Marls, which are now taken as the base.

Chalk is a weak rock and as such it should be noted that the drilling, excavating and sampling process is detrimental to its fabric and structure. Chalk samples that have been machine sampled will therefore appear to be of a lower descriptive grade, as given in CIRIA C574, than the in-situ chalk. Soil sampling and visual observations from open excavations i.e. trial pits would allow chalk description and classification in accordance with CIRIA 574 including commenting for any aperture and discontinuities in the chalk structure.

Erosional features, such as pipes, swallow holes and solution cavities, usually in-filled with drift deposits, are occasionally found in the chalk, sometimes manifesting themselves at the surface as shallow circular depressions. Solution features may be reactivated by the concentrated ingress of water from leaking drains or soakaways. Reactivation may lead to surface collapse.

Section 2 Site Works

2.1 Proposed Project Works

The proposed intrusive investigation was designed to provide information on the ground conditions and to aid the design of soakaways for the proposed development. The intended investigation, as outlined within the Soils Limited quotation (Q28823, dated 25th April 2024 and email correspondence on 16th May 2024), was to comprise the following items:

- UXO watching brief;
- Exploratory holes were scanned with a CAT and GENNY prior to excavation/drilling;
- 1 No. machine-excavated trial pit;
- 1 No. infiltration test within the trial pit;
- 1 No. windowless sampler location with monitoring well installation;
- 3 No. groundwater monitoring visits.

2.1.1 Actual Project Works

The actual project works were undertaken on 23rd and 24th May 2024, with subsequent sample logging and reporting. The actual works comprised:

- UXO watching brief;
- Exploratory holes were scanned with a CAT and GENNY prior to excavation/drilling;
- 1 No. machine-excavated trial pit (TP1);
- 1 No. infiltration test within the trial pit;
- 2 No. windowless sampler boreholes (WS1-WS2);
- 1 No. monitoring well installation in WS1;
- 3 No. groundwater monitoring visits.

The trial pit was backfilled with arisings and mounded to allow for subsequent settlement. Windowless sampler borehole WS1 was backfilled with gravel and bentonite following the installation of a monitoring well and WS2 was backfilled with gravel.

The installation details are as follows:

- Ground level to 1.00m bgl – plain 25mm internal diameter pipework/bentonite backfill;
- From 1.00m bgl to a depth of 4.00m bgl perforated 25mm internal diameter pipework and gravel backfill with bentonite seal at depth.

All exploratory hole locations are presented in Figure 3.

2.2 Ground Conditions

The exploratory holes were undertaken at suitable locations selected by the client. Engineering exploratory hole logs recording the ground conditions encountered are presented in Appendix B.1.

The maximum depths of exploratory holes have been included in Table 2.1.

Table 2.1 Final Depth of Exploratory Hole

Exploratory Hole	Depth (m bgl)
WS1	4.00 (w)
WS2	2.00
TP1	2.60

Notes: w = well installation

The soil conditions encountered were recorded and soil sampling commensurate with the purposes of the investigation was carried out. The depths given on the exploratory hole logs and quoted in this report were measured from ground level.

The soils encountered from immediately below ground surface have been described in the following manner. Where the soil incorporated an organic content such as either decomposing leaf litter or roots or has been identified as part of the in-situ weathering profile, it has been described as Topsoil both on the logs and within this report. Where man has clearly either placed the soil, or the composition altered, with say greater than an estimated 5% of a non-natural constituent, it has been referred to as Made Ground both on the log and within this report.

For more complete information about the soils encountered within the general area of the site reference must be made to the detailed records given within Appendix B, but for the purposes of discussion, the succession of conditions encountered in the exploratory holes in descending order can be summarised as:

Made Ground/Topsoil (MG/TS)
Head (HEAD)
Tarrant Chalk Member (not encountered.)

The ground conditions encountered in the exploratory holes are summarised in Table 2.2.

Table 2.2 Ground Conditions

Strata	Depth Encountered (m bgl)		Typical Thickness (m)	Typical Description
	Top	Bottom		
TS	GL	0.15	0.15	Soft dark brown slightly sandy SILT. Sand is predominantly medium. Frequent rootlets.
MG	GL	0.70	0.70	Turf over soft dark grey brown gravelly silty CLAY, with occasional fine brick fragments.

Strata	Depth Encountered (m bgl)		Typical Thickness (m)	Typical Description
	Top	Bottom		
HEAD	0.15-0.70	4.00[1,2]	Not proved	Soft light grey off white mottled brown slightly sandy very gravelly CLAY. Rare rootlets. Frequent angular to sub-angular fine to coarse to boulder flint gravel

Note(s): [1] Final depth of exploratory hole. [2] Base of strata not encountered. The depths given in this table are taken from the ground level on-site at the time of investigation.

Section 3 Groundwater

3.1 Groundwater Observations

Groundwater was not observed within the exploratory holes during excavation/drilling.

Monitoring of groundwater was undertaken within the standpipe installed in WS1 to a depth of 4.00m bgl on 3No. separate occasions on a fortnightly basis, as agreed with the client.

The results of the groundwater monitoring are provided within Table 3.1.

Table 3.1 Groundwater Observations

Date	Exploratory Hole	Depth of water (m bgl)	Base of Well (m bgl)
07/06/2024	WS1	DRY	4.00
26/06/2024		DRY	4.00
10/07/2024		DRY	4.00

Changes in groundwater level occur for a number of reasons including seasonal effects and variations in drainage.

The investigation was conducted in May (2024) when groundwater levels are near their annual maximum (highest) elevation, which typically occurs around March; the groundwater monitoring was undertaken between June and July 2024, when groundwater levels should be falling from their annual highest and approaching their minimum (lowest) elevation typically expected around September.

It is important to consider that the UK had its wettest spring since 1986 and the sixth wettest on record since 1836 (source: Met Office).

Section 4 Infiltration Testing

4.1 Infiltration Testing

Infiltration testing was performed within TP1 to provide guidance on the suitability of the ground for the adoption of a surface water drainage system.

The test comprised piping fresh water via a water tanker into the open trial hole, the drop in water level over time was then recorded to give an indication of soakage potential. BRE DG365:2016 states that for an accurate infiltration rate to be obtained a soakage pit needs to be filled three times in quick succession, after the head loss is at or exceeded 75%.

Due to relatively slow soakage, a single test cycle was possible to be undertaken within the testing period. It must be noted furthermore that the head loss did not reach 75%, therefore the test was not in full compliance with the BRE DG365:2016 guidance.

4.2 Test Results

A summary of the infiltration test undertaken in TP1 is given in Table 4.1 and the details of the test are given in Appendix C.1.

Table 4.1 – Infiltration Test Results

Exploratory Hole	Depth (m bgl)	Test Cycle	Water Depth (m bgl)		Duration (mins)	Infiltration Rate (m/sec)
			Start	End		
TP1	2.60	1	1.20	1.97	300	2.01E-05*
				2.25**	700**	5.90E-06**

Notes:
* Assuming 75% head loss equals actual depth of water of 1.97m bgl by end of the test after 300mins
** After extrapolating time data to circa 700 mins and assuming a 75% head loss of 2.25m bgl was achieved

4.3 Summary and Conclusion

Due to relatively slow drainage, an infiltration rate of 2.01E-05m/sec was initially calculated assuming the depth of water recorded at the end of the test would equal 75% head loss of water. Data was subsequently extrapolated to derive an approximation of the time required to achieve a 75% head loss, based on which a reduced infiltration rate of 5.90E-06m/sec was calculated, which was considered more appropriate and representative of the underlying ground conditions and associated drainage recorded.

Subject to drainage requirements, it was therefore considered that soakaways may not work satisfactorily given the relatively poor infiltration that was attributed to the predominantly cohesive thus low permeable nature of the soils of the Head encountered.

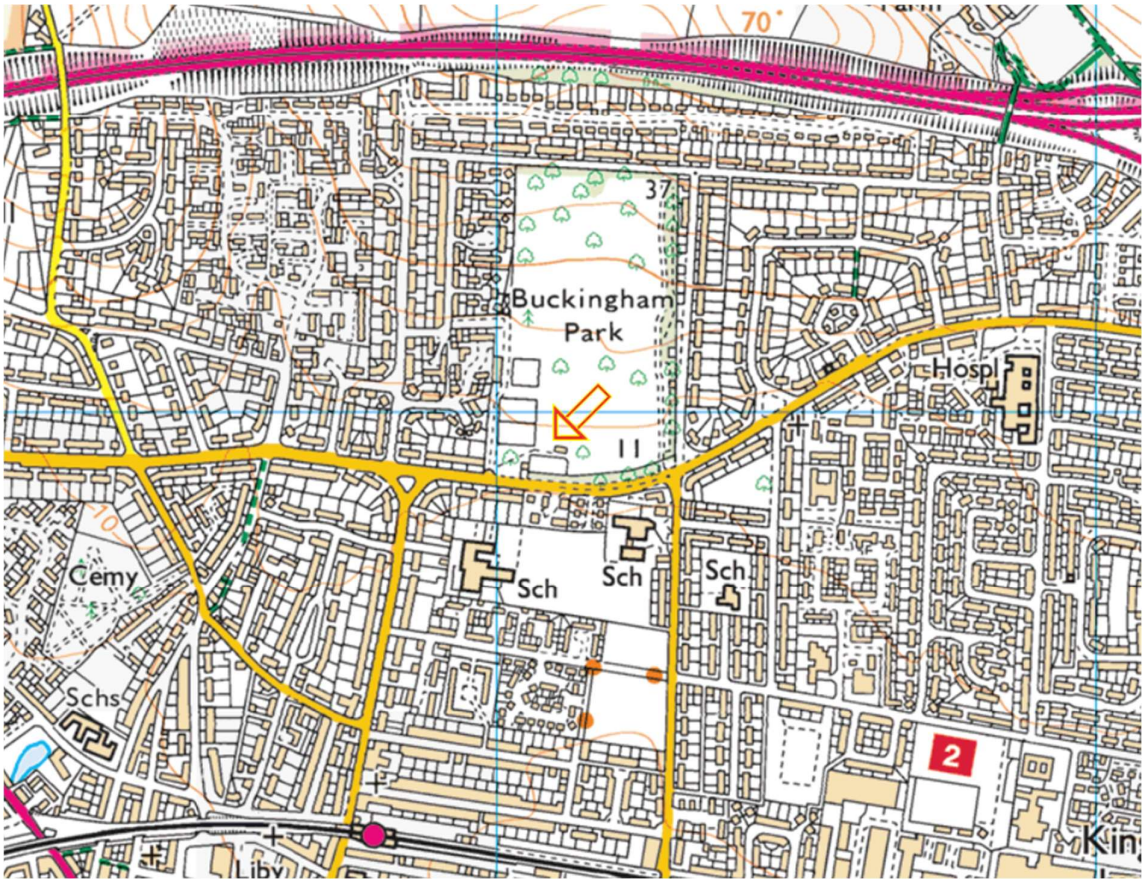
It is strongly recommended that the results of the in-situ permeability testing are passed to a drainage engineer for commentary and design. Consultation with the Environment Agency must be sought regarding any use which may have an impact on groundwater resources.

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Figure 1 – Site Location Map

Job Number 21463	**Project** South Pavilion, Buckingham Park, Upper Shoreham Road, Shoreham by Sea, West Sussex BN43 6BA
Client Price & Myers LLP	**Date** August 2024



Figure 2 – Aerial Photograph

Project
South Pavilion, Buckingham Park, Upper Shoreham Road, Shoreham by Sea, West Sussex, BN43 6BA

Client
Price & Myers LLP

Date
August 2024

Job Number
21463

N



Figure 3 – Exploratory Hole Plan

Project
South Pavilion, Buckingham Park, Upper Shoreham Road, Shoreham by Sea, West Sussex, BN43 6BA

Client
Price & Myers LLP

Date
August 2024

Job Number
21463

N

Appendix A Standards and Resources

The site works, soil descriptions and geotechnical testing was undertaken in accordance with the following standards were applicable:

- BS 5930:2015
- BS EN ISO 14688-1:2018 - Geotechnical investigation and testing - Identification and description
- BS EN ISO 14688-2:2018 - Geotechnical investigation and testing - Principles for a classification
- BRE Digest 365: Soakaway Design: 2016
- Google Earth
- British Geological Survey Website & iGeology App

Appendix B Field Work

Appendix B.1 Engineers Logs

Soils Limited
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Tel: 01737 814221 Email: admin@soilslimited.co.uk

Trial Pit Log

Trial Pit No. **TP1** Sheet 1 of 1

Project Name: Buckingham Park	Project No.: 21463	Method:	Hole Type: TP
Location: Upper Shoreham Road, Shoreham by Sea, BN43 6BA		Plant: 12tn machine	Scale: 1:25
Client: Price & Myers LLP	Trial Pit Length: 2.30m	Support: Trial Pit Width: 0.50m	Logged By
Dates: 24-05-2024	Level:	Co-ords:	

Water Strike	Samples & In Situ Testing: Depth	Type	Results	Depth (m)	Level (mAOD)	Legend	Stratum Description
							Turf over soft dark grey brown gravelly silty CLAY. Frequent rootlets. Occasional fine brick fragments. Occasional fine to coarse angular to sub-angular flint gravel. MADE GROUND
				0.70			Soft light grey off white mottled brown slightly sandy very gravelly CLAY. Rare rootlets. Frequent fine to coarse to boulder angular to sub-angular flint gravel. HEAD
				1.30			Soft light orange brown slightly sandy gravelly CLAY. Occasional fine to coarse to Boulder angular to sub-angular flint gravel. HEAD
				2.60			End of Pit at 2.600m

1 2 3 4 5

General Remarks:
No groundwater encountered.

Groundwater Remarks:

Sample Type
D: Disturbed
B: Bulk
J: Jar
W: Water

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Contract Name:		Client:				Hole ID:
Buckingham Park		Price & Myers LLP				**WS1**
Contract Number:	Start and End Date:	Logged By:	Checked By:	Status:		Hole Type:
21463	23-05-24	EF		FINAL		**WS**
Easting:	Northing:	Ground Level:	Plant Used:	Print Date:		Scale:
			Premier 3	26-06-2024		1:50

Weather: | Termination: | Sheet 1 of 1

Samples & In Situ Testing			Strata Details				Groundwater	
Depth	Type	Results	Level (mAOD)	Depth (m) (Thickness)	Legend	Strata Description	Water Strike	Backfill/ Installation
0.10	D			0.15		Soft dark brown slightly sandy SILT. Sand is predominantly medium. Frequent rootlets. TOPSOIL		
0.40	D			(0.35) 0.50		Firm brown, slightly sandy, slightly gravelly CLAY. Sand is medium to coarse. Gravel is angular to subangular fine to coarse flint. Frequent rootlets. HEAD		
0.90	D			(0.75)		Firm brown slightly gravelly CLAY. Gravel is angular to subangular fine to coarse flint. Occasional rootlets and rare lignite. HEAD		
1.40	D			1.25		Cream mottled light brown, sandy, slightly gravelly SILT. Sand is medium to coarse. Gravel is angular to subangular fine to coarse flint. HEAD		
1.80	D			1.50 (0.40)		Firm light grey, sandy, slightly gravelly CLAY. Sand is coarse. Gravel is angular to subangular fine to coarse flint. HEAD		
2.10	D			1.90		Firm brown mottled light brown, slightly gravelly CLAY. Gravel is subangular fine to coarse flint. HEAD		
2.70	D			(0.95) 2.85		Firm light brown sandy CLAY. Sand is medium to coarse. HEAD		
3.20	D			(0.45) 3.30		Firm light brown mottled cream, very sandy, slightly gravelly CLAY. Sand is predominantly coarse. Gravel is angular fine to coarse flint. HEAD		
3.50	D			(0.40) 3.70		Firm light brown sandy CLAY. Sand is predominantly coarse. HEAD		
3.80	D			(0.30) 4.00		*Firm light brown mottled cream, very sandy CLAY. Sand is predominantly coarse.* End of Borehole at 4.00m		

Start & End of Shift Observations

Date	Time	Depth (m)	Casing (m)	Water (m)

Borehole Diameter

Depth (m)	Dia (mm)

Casing Diameter

Depth (m)	Dia (mm)

Remarks:
Rootlets observed to 0.90m bgl.

Chiselling

From (m)	To (m)	Duration	Remarks

Installation

Top (m)	Base (m)	Type	Dia (mm)
0.00	1.00	PLAIN	
1.00	4.00	SLOTTED	

Water Strikes

Strike (m)	Casing (m)	Sealed (m)	Time (mins)	Rose to (m)	Remarks

Hand vane (HV), Hand penetrometer (HP) reported in kPa. PID reported in ppm.

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Contract Name:		Client:			Hole ID:
Buckingham Park		Price & Myers LLP			**WS2**
Contract Number:	Start and End Date:	Logged By:	Checked By:	Status:	Hole Type:
21463	23-05-24	GJB		FINAL	**WS**
Easting:	Northing:	Ground Level:	Plant Used:	Print Date:	Scale:
			Premier 3	26-06-2024	1:50

Weather: | Termination: | Sheet 1 of 1

Samples & In Situ Testing			Strata Details				Groundwater	
Depth	Type	Results	Level (mAOD)	Depth (m) (Thickness)	Legend	Strata Description	Water Strike	Backfill/ Installation
				0.15		Soft dark brown slightly sandy SILT. Sand is predominantly medium. Frequent rootlets. TOPSOIL		
				(1.85)		Gravelly sandy CLAY/ clayey gravelly SAND. HEAD		
				2.00		End of Borehole at 2.00m		

1 2 3 4 5 6 7 8 9 10

Start & End of Shift Observations					Borehole Diameter		Casing Diameter	
Date	Time	Depth (m)	Casing (m)	Water (m)	Depth (m)	Dia (mm)	Depth (m)	Dia (mm)

Chiselling				Installation			
From (m)	To (m)	Duration	Remarks	Top (m)	Base (m)	Type	Dia (mm)

Remarks:

Water Strikes					
Strike (m)	Casing (m)	Sealed (m)	Time (mins)	Rose to (m)	Remarks

Hand vane (HV), Hand penetrometer (HP) reported in kPa. PID reported in ppm.

Appendix C Infiltration Testing Data

Appendix C.1 Infiltration Data

Test Location: TP1

Soak Test Data

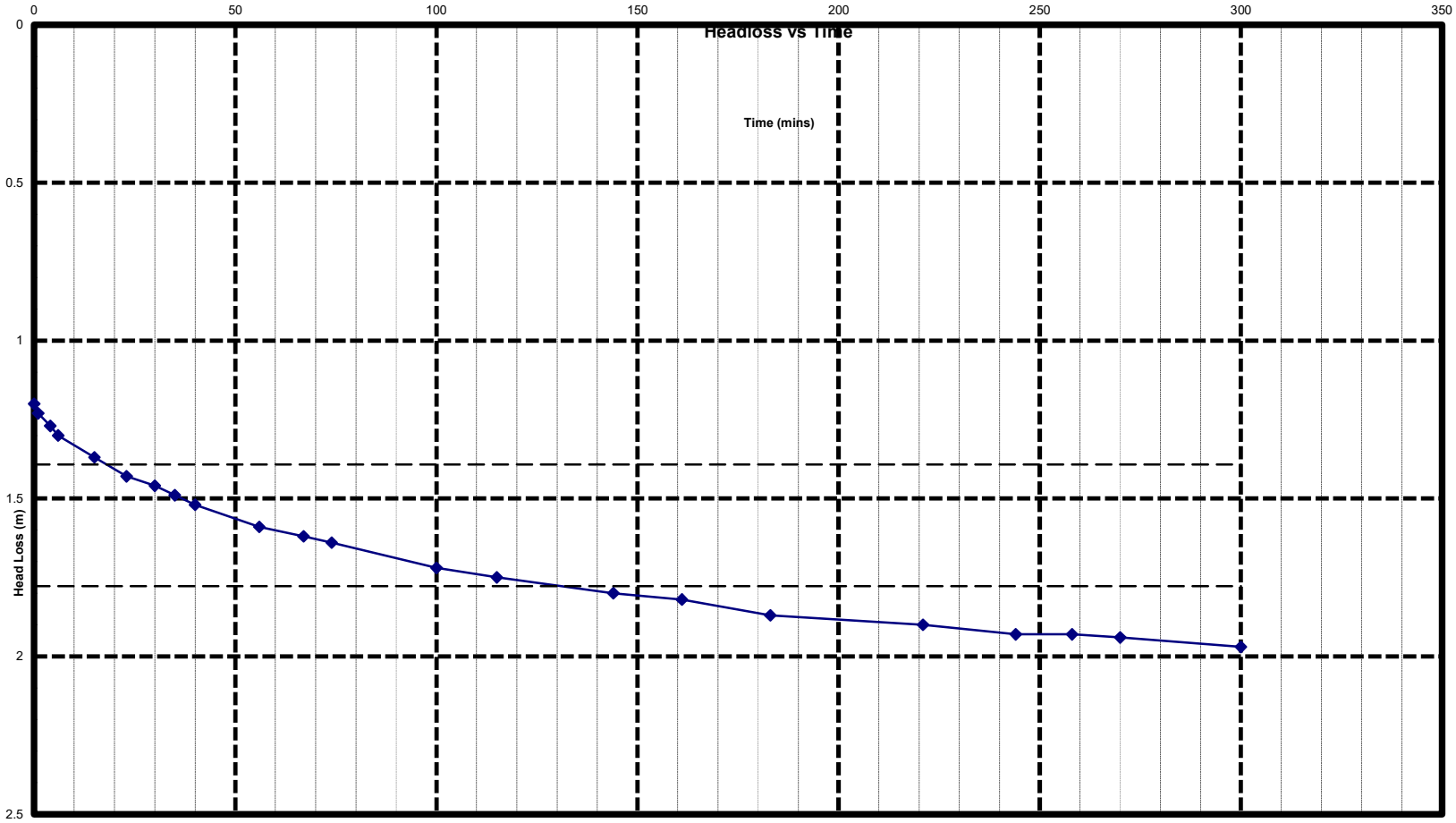
Site	Buckingham Park
Test number	1
Date	24-05-2024
Depth	2.600
Avg. Length	2.3
Avg. Width	0.5

Time (mins)	**Water Depth**
0	1.2
1.00	1.23
4.00	1.27
6.00	1.3
15.00	1.37
23.00	1.43
30.00	1.46
35.00	1.49
40.00	1.52
56.00	1.59
67.00	1.62
74.00	1.64
100.00	1.72
115.00	1.75
144.00	1.8
161.00	1.82
183.00	1.87

25% =	1.393	
75%=	1.778	
$V_{75>25}$ (m^3)	0.44275	
A_{50} (M^2)	3.306	
T_{25}	19	From Graph
T_{75}	130	From Graph
T	111	

f= 0.001206515 m/min

f= 2.01E-05 m/sec


Headloss vs Time
Time (mins)
Head Loss (m)
0
50
100
150
200
250
300
350
0
0.5
1
1.5
2
2.5

Test Location: TP1

Soak Test Data

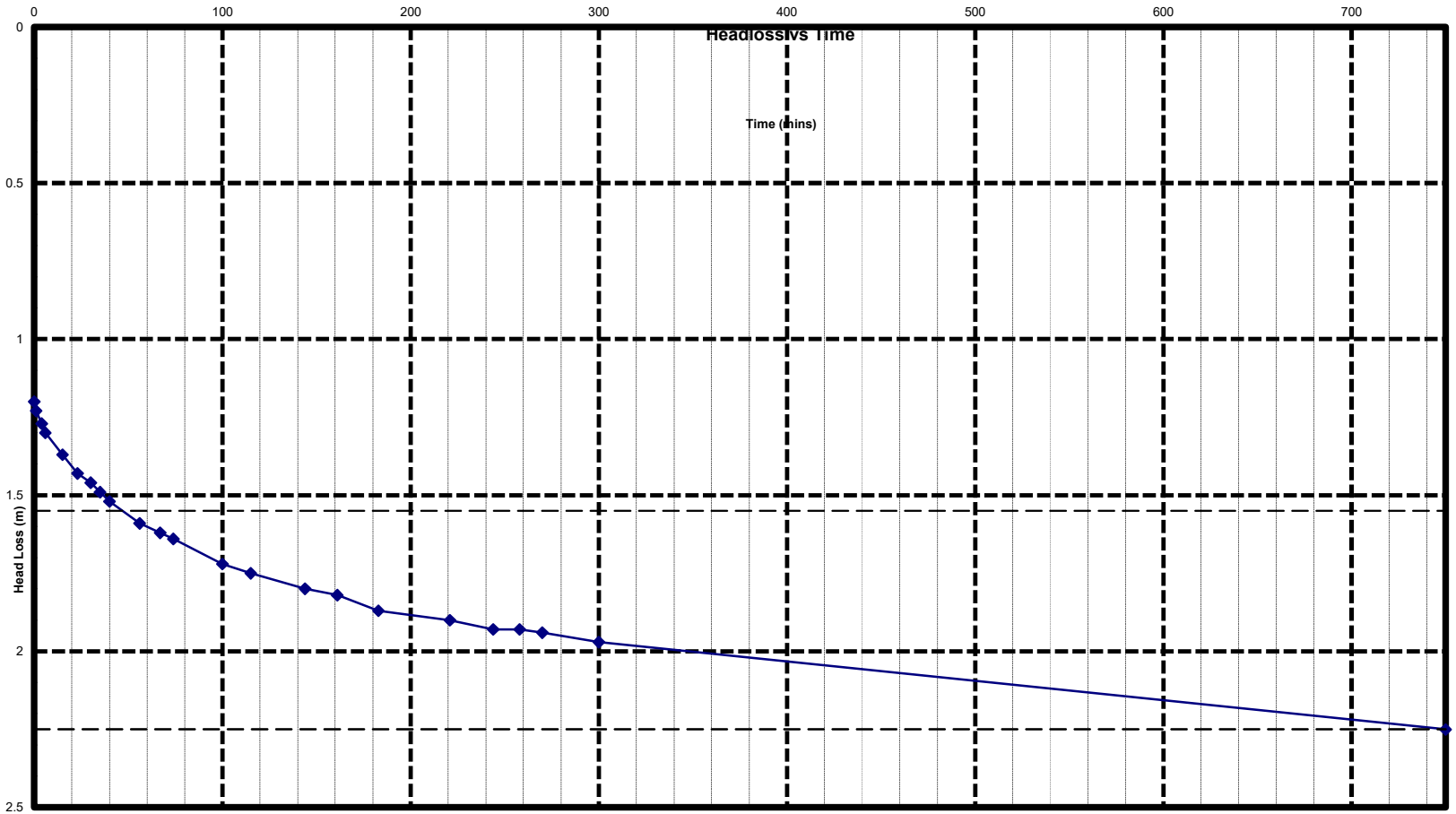
Site	Buckingham Park
Test number	1
Date	24-05-2024
Depth	2.600
Avg. Length	2.3
Avg. Width	0.5

Time (mins)	Water Depth
0	1.2
1.00	1.23
4.00	1.27
6.00	1.3
15.00	1.37
23.00	1.43
30.00	1.46
35.00	1.49
40.00	1.52
56.00	1.59
67.00	1.62
74.00	1.64
100.00	1.72
115.00	1.75
144.00	1.8
161.00	1.82
183.00	1.87

25% =	1.550	
75%=	2.250	
$V_{75>25}$ (m^3)	0.805	
A_{50} (M^2)	3.11	
T_{25}	19	From Graph
T_{75}	750	From Graph
T	731	

f= 0.000354094 m/min

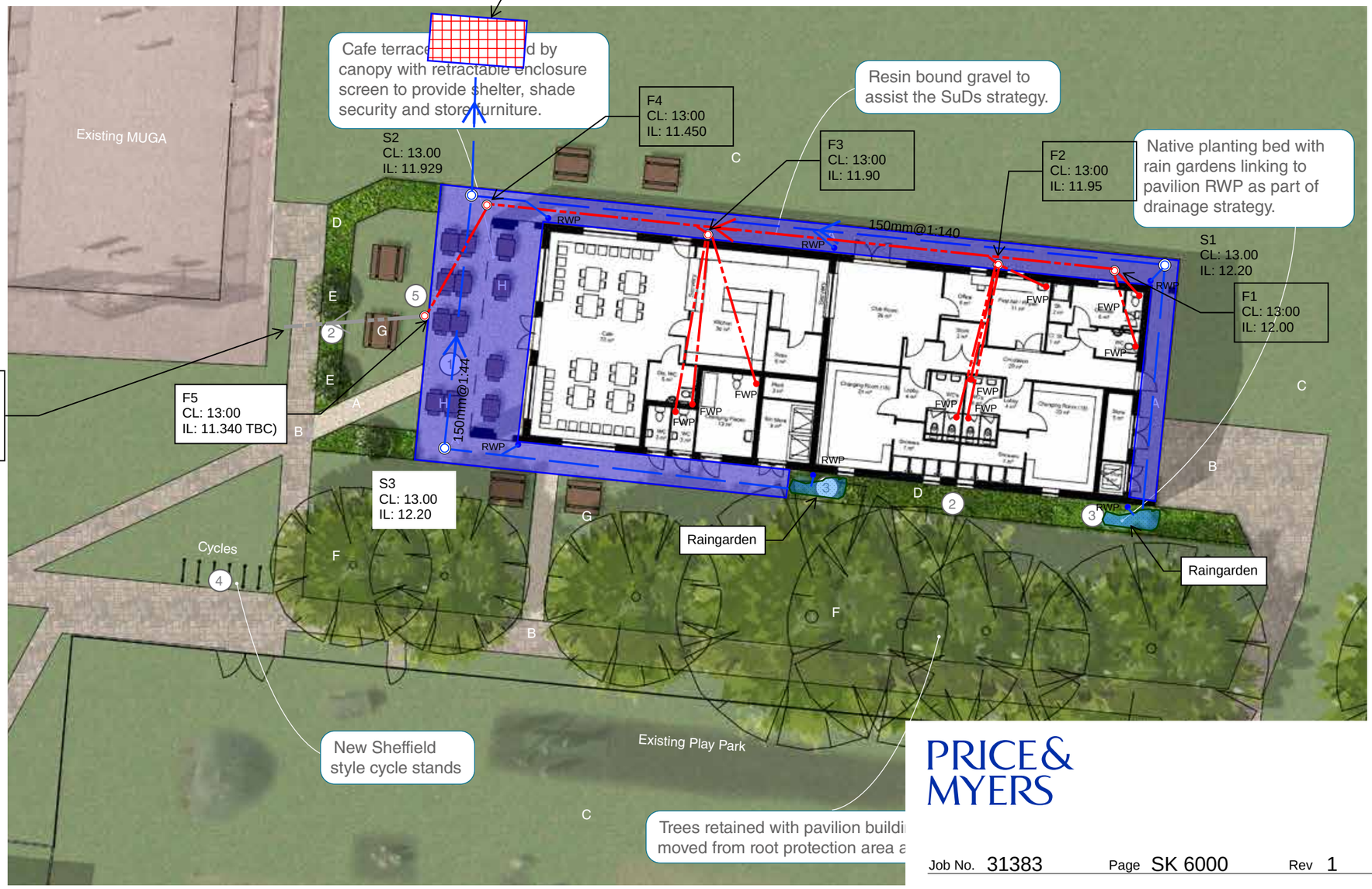
f= 5.90E-06 m/sec


Headlossvs Time
Time (mins)
Head Loss (m)
0
100
200
300
400
500
600
700
0
0.5
1
1.5
2
2.5

Appendix D Information Provided by the Client

KEY:

- Foul Water Drain
- Surface Water Drain
- Existing Foul Water Drain
- Foul Water Manhole
- Surface Water Manhole
- RWP — Rainwater Pipe
- FWP — Foul Water Pipe
- Resin Bound Permeable System



PRICE & MYERS

Job No.	31383	Page	SK 6000	Rev	1
Date	7/03/2024	Eng	DLin	Chd	
Job	Buckingham Park				

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